

## Experimental Investigation on Strength Characteristics of Concrete



### Engineering

**KEYWORDS:** Rice Husk Ash, Coir, Environmental Pollution, Compressive Strength, Splitting Tensile Strength, Flexural Strength and Durability.

**M. P. Karthik**

Asst.Prof. SSM Institute of Engineering and Technology, Dindigul

**Dr. V. Sreevidya**

Asso.Prof. Sri Krishna College of Technology, Coimbatore

### ABSTRACT

*From the investigation this paper express that the strength and durability characteristics of M40 grade concrete in which cement is partially replaced by rice husk ash and coir. Coconut fibers are reported as most ductile and energy absorbent material. It is concluded that coconut fibers have the potential to be used in composites for different purposes. Rice husk ash is obtained by burning rice husk in a controlled manner without causing environmental pollution. There is a good potential to make use of RHA as a valuable pozzolanic material to give almost the same properties as that of micro silica. In this study, the strength related properties such as compressive strength, splitting tensile strength, flexural strength were calculated and better durability characteristics when compared to other replacement levels.*

### INTRODUCTION

Concrete is one of the crucial materials for infrastructure development due to its versatile application, globally its usage is second to water. Due to increase in the cost of conventional building materials and environmental hazard, the designers and developers are looking for 'alternative materials' to reduce the use of cement in civil engineering constructions. For this objective, the researchers are trying to use various waste products in concrete technology. The objective of this investigation is to study the effect of partial replacement of cement by Rice husk Ash as a Mineral admixture in concrete and also adding Natural fiber (Coir) to increase the tensile strength of concrete.

### MATERIALS USED AND METHODOLOGY CEMENT

The Ordinary Portland Cement of 53 Grade conforming to IS 12269 – 1987 was used in this study. The specific gravity, initial and final setting of OPC 53 grade were 3.15, 30 and 600 minutes respectively.

### FINE AGGREGATE

Locally available river sand conforming to grading zone II of IS 383 –1970. Sand passing through IS 4.75mm Sieve will be used with the specific gravity of 2.65.

### COARSE AGGREGATE

Locally available blue metal was used. Crushed granite stones of size passing through 20mm sieve and retained on 4.75 mm sieve as per IS: 383-1970 was used for experimental purpose.

### WATER

Casting and curing of specimens were done with the potable water that is available in the college premises.

### RICE HUSK ASH

RHA, produced after burning of Rice husks (RH) has high reactivity and pozzolanic property. Indian Standard code of practice for plain and reinforced concrete, IS 456- 2000, recommends use of RHA in concrete but does not specify quantities. The physical and chemical properties of RHA are shown in Table 1 and Table 2.

**Table 1 Physical Properties of Rice Husk Ash**

Physical properties	Value
Specific gravity	2.19
Fineness passing through 45µm sieve in (%)	99.5
Colour	Grey

**Table 2 Chemical Properties of Rice Husk Ash**

Chemical properties	Value
Silicon dioxide(SiO <sub>2</sub> )	88.32
Silicon dioxide(SiO <sub>2</sub> )	0.46
Ferric oxide(Fe <sub>2</sub> O <sub>3</sub> )	0.67
Calcium oxide(CaO)	0.51
Magnesium oxide(MgO)	0.44
Sodium oxide(Na <sub>2</sub> O <sub>3</sub> )	0.12
Potassium oxide(K <sub>2</sub> O)	2.91

### COIR

Coconut fiber is one of the natural fibers abundantly available in tropical regions, and is extracted from the husk of coconut fruit. The aim of this review is to spread awareness of coconut fibers as a construction material in civil engineering. The versatility and applications of coconut fibers in different fields is discussed in detail. Coconut fibers are reported as most ductile and energy absorbent material. It is concluded that coconut fibers have the potential to be used in composites for different purposes.

In civil engineering, coconut fibers have been used as reinforcement in composites for non-structural components. There is a need of investigating the behavior of coconut fiber reinforced concrete to be used in main structural components like beams and columns.

**Table 3 Properties of Coconut Fiber**

Properties	Value
Fiber length (mm)	50-110
Fiber diameter (mm)	0.1-0.406
Average tensile strength(N/mm <sup>2</sup> )	150
Specific gravity	1.12-1.15
Elongation (%)	10-25

### EXPERIMENTAL INVESTIGATION

#### MIX DESIGN

Mix design is the process of selecting suitable ingredients of concrete and determines their relative proportions with the object of producing concrete of certain minimum strength and durability as economically as possible.

**Table 4 Design of Concrete Mix for M40 Grade**

CEMENT (Kg/m <sup>3</sup> )	F.A (Kg/m <sup>3</sup> )	C.A (Kg/m <sup>3</sup> )	WATER (Kg/m <sup>3</sup> )
492.5	755	968	197
1	1.53	1.96	0.4

**TEST PROCEDURE**

The experimental investigations carried out on the test specimens to study the strength-related properties of concrete using Rice husk ash and coir. Here, an attempt was made to study the strength development at different replacement levels at different ages with Rice husk ash and coir the results were compared.

Concrete were produced with 10, 12.5 and 15% of the RHA as cement replacement (in mass) and coir is replaced with 1, 2, and 3% (by mass). Totally thirteen different proportions of concrete mixes are used. Ratio for M40 Grade as per IS 10262:2009.

**CASTING AND TESTING OF SPECIMENS**

All the ingredients were first mixed in dry condition in the concrete mixer. The concrete mix proportion is already shown in table. The calculated amount of water added to the dry mix and mixed thoroughly to get uniform mix. Before casting machine oil was smeared on the inner surface of the mould and the concrete was poured in to the mould. After 24 hours of casting, the specimens were demoulded and cured for 28 days using water tank. After the curing period was over, the specimens were white washed and kept ready for testing. For each mix, six cube specimens, three cylinder specimens and three beam specimens of size 100 x 150 x 1000 mm. Cube and specimens were tested on 7 days and 28 days.

**DURABILITY CHARACTERISTICS**

To trace the history of concrete, more attention has been given to the aspect of its strength. Concrete performance has been specified and evaluated in terms of its compressive strength. Hence, in this research, experimental study was carried out on the test specimens to ascertain the durability- related property such as acid resistance of the designed M40 grade. The acid resistance tests were carried out on 150 mm size cube specimens at the age of 28 days curing. The cube specimens were weighed and immersed in water diluted with one percent by weight of sulphuric acid for 28 days continuously. Then the specimens were taken out from the acid water and the surfaces of the cubes were cleaned. Then, the weight of the specimens was found out and the average percentage of loss of weight was calculated.

**FLEXURAL STRENGTH TEST**

The size of beam used was 100 x 150 x 1000 mm. Three cubes of size 150 x 150 x150 mm were cast as control specimens. M40 grade is attempted, with different replacement levels of cement with of RHA. To start with specimens having a span of 1000 mm were tested under two-point loading. The effective span was 800mm. Hence the point load was applied at one third point from the end supports.

The size of the beam was 100 x 150 x 1000 mm. The reinforcement used were high yield strength deformed (HYSD) bars 2 No's of 10mm diameter in the tension side and 2 No's of 8 mm diameter in compression zone and three specimens for each parameter. The shear reinforcement is designed in such a way that, the shear capacity of the specimen is higher than the flexural strength. This is done to ensure flexural failure. For shear span 2 legged 6mm diameter stirrups at 120mm center to center is provided as shown in Fig1.



**Fig :1 Reinforcement Details**

**Table 5 Specimen Details**

SL.NO	Beam designation	RHA	COIR	No of Specimens
1	NM+10%+1%	10	1	1
2	NM+10%+2%	10	2	1
3	NM+10%+3%	10	3	1
4	NM+12.5%+1%	12.5	1	1
5	NM+12.5%+2%	12.5	2	1
6	NM+12.5%+3%	12.5	3	1
7	NM+15%+1%	15	1	1
8	NM+15%+2%	15	2	1
9	NM+15%+3%	15	3	1

**RESULT AND DISCUSSION**

**STRENGTH CHARACTERISTICS**

**Cube Compressive Strength**

The test results was observed that the maximum compressive strength is obtained for mix with 12.5% RHA and 1% COIR was observed that the maximum compressive strength at the water-binder ratio of 0.40. The compressive strength development is due to the pozzolanic reaction of RHA. The rapid rate of strength development is due to the fact that for lower water-binder ratio, the cement particles are held at closer interval than for higher water-binder ratios. Also due to the action of silica fume on calcium hydroxide, more gel is formed. These two factors enhance the formation of a continuous system of gel, which provides better development of strength at early ages since, silica fume starts react with calcium hydroxide and produces C-S-H gel immediately.

**Cylinder Split Tensile Strength**

The split tensile strength results of mixes at the age of 28 days for different replacement levels such as 10%, 12.5%, and 15% of Cement with Rice husk ash and 1%, 2% and 3 % replacement of coir are presented in Tables 6.3 and 6.4. The development of Compressive Strength with ages for the above different mixes was plotted in the form of graphs as shown in Fig 2 and 3.

From the test results it was observed that the maximum split tensile strength is obtained for mix with 12.5% RHA. In the replacement of RHA the mix with 12.5% RHA and 2% COIR was observed that the maximum split tensile strength at the water-binder ratio of 0.40.

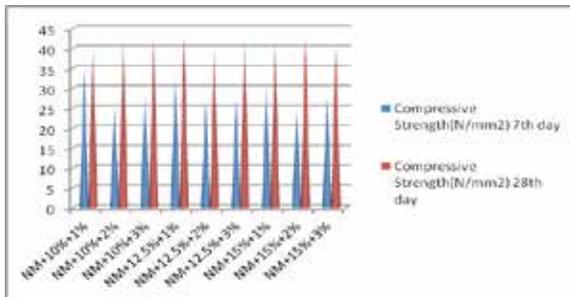
**Flexure Strength**

The size of beam used was 100 x 150 x 1000 mm. Three cubes of size 150 x 150 x150 mm were cast as control specimens.

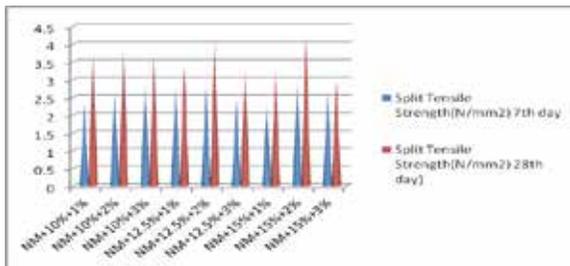
To start with trial specimens having a span of 1000 mm were tested under two-point loading. The effective span was 800 mm. Hence the point load was applied at one third point from the end supports.

**Table 6 Results of Compressive & Split Tensile Test**

Mix	RHA (%)	Coir (%)	Compressive Strength(N/mm <sup>2</sup> ) 7 <sup>th</sup> day	Compressive Strength(N/mm <sup>2</sup> ) 28 <sup>th</sup> day	Split Tensile Strength(N/mm <sup>2</sup> ) 7 <sup>th</sup> day	Split Tensile Strength(N/mm <sup>2</sup> ) 28 <sup>th</sup> day
NM+10%+1%	10	1	34.82	40.88	2.33	3.81
NM+10%+2%	10	2	25.48	42.65	2.65	3.95
NM+10%+3%	10	3	28.42	42.18	2.88	3.65
NM+12.5%+1%	12.5	1	33	43.23	2.78	3.42
NM+12.5%+2%	12.5	2	26.55	40.11	2.8	4.12
NM+12.5%+3%	12.5	3	27.11	42.65	2.44	3.29
NM+15%+1%	15	1	31.25	41.32	2.32	3.21
NM+15%+2%	15	2	25.3	42.89	2.95	4.2
NM+15%+3%	15	3	27.62	40.56	2.6	3.02



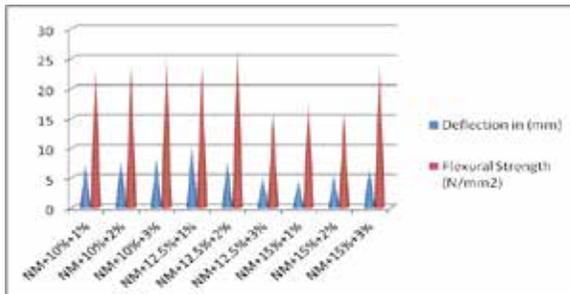
**Fig 2 Comparison of Compressive Strength at 7 & 28th Day**



**Fig. 3 Comparison of Split Tensile**

**Table 7 Flexural Strength Result of Beams**

Mix	RHA (%)	COIR (%)	Ultimate Load(N/mm <sup>2</sup> )	Deflection in (mm)	Flexural Strength (N/mm <sup>2</sup> )
NM+10%+1%	10	1	52.8	7.05	23.47
NM+10%+2%	10	2	52.8	7.88	23.47
NM+10%+3%	10	3	57.2	8.03	25.42
NM+12.5%+1%	12.5	1	52.8	10.2	23.47
NM+12.5%+2%	12.5	2	61.6	7.52	27.38
NM+12.5%+3%	12.5	3	35.2	5.02	15.65
NM+15%+1%	15	1	39.6	4.36	17.6
NM+15%+2%	15	2	35.2	5.28	15.65
NM+15%+3%	15	3	52.8	6.54	23.47



**Fig. 4 Comparison of Flexural & Deflection Results**

**DURABILITY CHARACTERISTICS**

**ACID RESISTANCE**

The acid resistance tests were carried out on 150 mm size cube specimens at the age of 28 days curing. The cube specimens were weighed and immersed in water diluted with one percent by weight of sulphuric acid for 28 days continuously. Then the specimens were taken out from the acid water and the surfaces of the cubes were cleaned. Then, the weight of the specimens was found out and the average percentage of loss of weight was calculated.

**SPECIMENS UNDER ACID RESISTANCE TEST**



**Fig.5 View of Acid Resistance Test**

**BEHAVIOUR OF SPECIMENS AFTER ACID RESISTANCE TEST**



**Fig.6 View of Specimen after Acid resistance Test**

**Table 8 Acid Resistance Test Results**

Mix	Reduction in weight (%)
NM+10%+1%	1.18
NM+10%+2%	1
NM+10%+3%	0.89
NM+12.5%+1%	0.82
NM+12.5%+2%	0.8
NM+12.5%+3%	0.75

NM+15%+1%	1.45
NM+15%+2%	1.38
NM+15%+3%	1.35

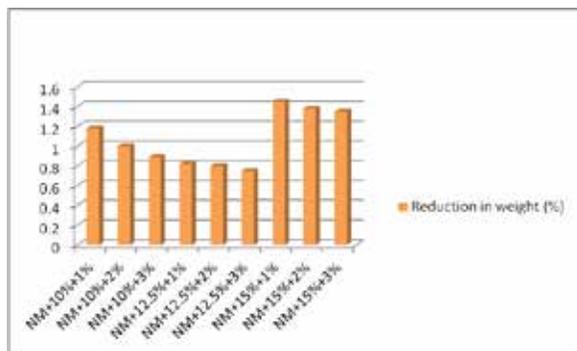


Fig.7 Comparison of Acid Resistance Test Results

**CONCLUSION**

**Cube Compressive Strength**

At the age of 28days the compressive strength of mix 12.5%RHA + 1%COIR shows the highest strength when compared to other replacement levels of RHA with Cement. This indicates that the optimum percentage of replacement of Rice husk ash with cement is 12.5 percent

The Rice husk ash used in this investigation exhibits good pozzolanic properties. Therefore, it is strongly recommended for the production of concrete.

**Split Tensile Strength**

In the replacement levels of Rice husk ash with cement, the optimum replacement of Rice husk ash with cement for M40 grade of concrete was found to be 12.5% for achieving maximum split tensile strength at the age of 28 days.

The tensile strength increases along with increase in compressive strength. The tensile strength of concrete is 6 to 9 percent of cube compressive strength.

**Experimental Investigations on Flexural Behaviour of Concrete**

At the age of 28days the flexural strength of mix 12.5% RHA + 2% COIR shows the highest strength when compared to other replacement levels of Rice husk ash with cement. The ultimate load and first crack load for the above mix shows the highest value and this indicates that the optimum percentage of replacement of Rice husk ash with cement is 12.5 percent.

**Experimental Investigations on Durability Characteristics of Concrete**

Based on the experimental investigations carried out on the durability characteristics of concrete mixes, the following conclusions are arrived at:

**Acid Resistance**

The acid resistance of concrete mixes containing Rice husk ash was higher when compared with that of the concrete mixes without Rice husk ash at the age of 28 days.

In the replacement levels of Rice husk ash with cement mix 12.5% RHA+ 3% coir shows the optimum weight reduction when compared to other replacement levels. This indicates that the optimum percentage of replacement of Rice husk ash with cement is 12.5 percent. When the replacement levels increase the effect of acid is more.

**REFERENCE**

1. IS 456-2000, "Plain and Reinforced Concrete-Code of Practice", Bureau of Indian Standards,(BIS 2000). Fifth Reprint, 2002. | 2. IS 516-1959, "Methods of tests for strength of concrete", Bureau of Indian Standards, 1992. | 3. IS 10262-2009, "Concrete Mix Proportioning-Guidelines", Bureau of Indian Standards,(BIS 2009), First Revision, 2009 | 4. Maged S. Sobhy and M. T. Tammam (2010), "The Influence of Fiber Length and | Concentration on the Physical Properties of Wheat Husk Fibers Rubber Composites", ID 528173, pp. 583-591. | 5. Muthadhi A., R. Anita and Dr. S.Kothandaraman (2007), "Rice Husk Ash - Properties and its Uses: A Review", Journal of the Institution of Engineers, India. Civil Engineering Division, vol. 88, pp. 50-56. | 6. Pravin V Domke, Sandesh D Deshmukh, Satish D kene and R.S.Deotale (2009), "Study of Various characteristic of concrete with Rice Husk Ash as a partial replacement of cement with natural fibers (Coir)", International Journal of Engineering, Vol. 1, Issue 3, pp.554-562. | 7. Rahmat madandoust, Malec Mohammed Ranjibar, Hamed Ahmadi Moghadam and Syed Yasin Mousari (2011), "Mechanical properties and durability assesment of rice husk ash concrete", Biosystem, Elsevier Journals Ltd., Vol.30, pp. 1589-1598. | 8. R. Sathish Kumar (2012), "Experimental study on the properties of concrete made with alternate construction materials", International Journal of Modern Engineering Research, Vol. 2, pp-3006-3012. | 9. Utsev, J. T. and Taku, J. K. (2012), "Coconut Shell Ash As Partial Replacement of Ordinary Portland Cement In Concrete Production", International Journal of Scientific & Technology Research Vol.1, pp. 86-89. | 10. Vanchai seta, Chai jaturapitakul and Kraiwod Kittikomol (2007), "Influence of pozzolana from various by-product materials on mechanical properties of high strength concrete", Construction and Building Materials, Elsevier Journals Ltd., Vol.30,pp.1589-1598. | 11. Weerachart Tangchiapat, Rak Buranasing, Chai Jaturapitakul and Prinya Chindaprasirt (2008), "Influence of rice husk ash - bark ash on mechanical properties of concrete containing high amount of recycled aggregate", Construction and Building Materials, Elsevier Journals Ltd., Vol.30, pp. 1812-1819. |