

Summerization of Effect of Irregularity and Initial Stresses with Normal and Tangential Loading in Elastic Medium



Mathematics

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ABSTRACT

In the present paper, we have summarized the effect of presence of rectangular irregularity on displacements and stresses due to normal and tangential loading in transversely isotropic elastic medium and initially stressed orthotropic elastic medium. Due to their closeness to natural environmental conditions, irregular boundaries on the elastic medium have gained much importance in the study of elastic deformation field.

I. Introduction

The solution of problem of the deformation of elastic materials due to loading has been finding wide applications in geophysics, soil mechanics and engineering when the source surface is very long in on direction in comparison to the others, the use of two-dimensional is justified and consequently calculations are simplified to a great extent and we gets a closed form analytical solution. A very long strip-source and a very long line source are examples of such two-dimensional sources. The deformation due to loading such as inclined line load, strip-load, continuous line load, etc., is useful in analyzing the field around mining tremors and drilling in to the crust of the earth. It can also contribute to the theoretical consideration of the seismic and volcanic sources, since it can account for the deformation fields in the entire volume surrounding the source region.

Kuo [11] studied the static response of a stratified elastic half space due to surface loads. He used Thomson-Haskell matrix method which was earlier used in elastodynamics. Garg et al. [17] discussed the general plain- strain problem of an infinite unstressed orthotropic elastic medium due to 2-D sources. They obtained the deformation due to inclined line load by using eigen value approach with distinct eigen values. By considering distinct eigen values Selim and Ahmed [15] used same eigen value approach and obtained closed form analytical expression for displacement and stresses at any point of initially stressed orthotropic elastic medium due to an inclined line load.

A transverse isotropic has an axis of cylindrical symmetry. This is usually the result of parallel crack with coplanar normal or aligned grains. Pan [7] provided unified solution of the static deformation of the transversely isotropic and layered elastic half space by seismic sources. Singh et al.

[12] obtained that in a transversely isotropic elastic medium and in an isotropic elastic medium the eigen values become equal and Chugh et al. [19] obtained the corresponding deformation field in isotropic [unstressed] elastic medium.

The problem of static deformation with irregularity present in the elastic medium due to continental margin, mountain roots etc. is very important to study. The problem of irregular thickness of the medium was discussed by Sato [20], De Noyer [10], Mal [3], Kar et al. [4], Chhattopadhyay et al. [2], Chhattopadhyay and Pal [1] and others. Selim [14] studied the two dimensional static problem of an isotropic elastic half space with irregularity present in the medium. Madan et al. [5], [6] obtained the closed form expressions on the deformation fields at any point of irregular transversely isotropic elastic medium. In order to understand better the behavior of media at continental margins the static deformation with irregular thickness under initial stress is very important. Since, in fact the earth is an initially stressed medium. Tolstoy [1982] derived explicit solutions of the dynamical equations for a pre-stressed solid under horizontal compression is a gravity field. Selim and Ahmed [15] discuss the plane strain problem of initially stressed orthotropic elastic medium. Here we have reproduced collectively displacements and stresses due to normal and tangential loading in irregular transversely isotropic elastic medium and irregular initially stressed orthotropic elastic medium earlier discussed by Madan et al. [5], [6] and Selim [16].

II. Basic Equations

(a) Unstressed Elastic Medium

The stress strain relations in matrix form for a medium with hexagonal or transverse isotropic elastic symmetry is

$$\begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{33} \\ \sigma_{23} \\ \sigma_{13} \\ \sigma_{12} \end{bmatrix} = \begin{bmatrix} c_{11} & c_{12} & c_{13} & 0 & 0 & 0 \\ c_{12} & c_{11} & c_{13} & 0 & 0 & 0 \\ c_{13} & c_{13} & c_{33} & 0 & 0 & 0 \\ 0 & 0 & 0 & c_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & c_{44} & 0 \\ 0 & 0 & 0 & 0 & 0 & \frac{c_{11}-c_{12}}{2} \end{bmatrix} \begin{bmatrix} e_{11} \\ e_{22} \\ e_{33} \\ 2e_{23} \\ 2e_{13} \\ 2e_{12} \end{bmatrix} \tag{1}$$

where the two-suffix quantity c_{ij} denotes the elastic constants of the medium. So in transversely isotropic elastic medium we have five elastic constants. For an elastic isotropic medium, these constants reduce to just two as given below

$$\begin{aligned} c_{11} &= c_{33} = \lambda + 2\mu, \\ c_{12} &= c_{13} = \lambda, \\ c_{44} &= \frac{c_{11}-c_{12}}{2} = \mu \end{aligned} \tag{2}$$

where λ and μ are the Lamé's constants.

In the absence of body forces, the equilibrium equations in the Cartesian coordinate system (x,y,z) are

$$\sigma_{iij} = 0 \tag{3}$$

Where σ_{ij} ($i, j=1,2,3$) are the stress components for transversely isotropic medium. The strain displacement relations are

$$e_{ij} = e_{ji} = \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right), \quad 1 \leq i, j \leq 3,$$

where $(u_1, u_2, u_3) = (u, v, w)$ and $(x_1, x_2, x_3) = (x, y, z)$.

The equilibrium equations in terms of displacement components obtained from (1)-(4) are

$$\begin{aligned} c_{11} \frac{\partial^2 u}{\partial x^2} + \left(\frac{c_{11}-c_{12}}{2} \right) \frac{\partial^2 u}{\partial y^2} + c_{44} \frac{\partial^2 u}{\partial z^2} + \left(\frac{c_{11}+c_{12}}{2} \right) \frac{\partial^2 v}{\partial x \partial y} + (c_{13} + c_{44}) \frac{\partial^2 w}{\partial x \partial z} &= 0 \\ \left(\frac{c_{11}-c_{12}}{2} \right) \frac{\partial^2 v}{\partial x^2} + c_{11} \frac{\partial^2 v}{\partial y^2} + c_{44} \frac{\partial^2 v}{\partial z^2} + \left(\frac{c_{11}+c_{12}}{2} \right) \frac{\partial^2 u}{\partial x \partial y} + (c_{13} + c_{44}) \frac{\partial^2 w}{\partial y \partial z} &= 0 \end{aligned}$$

$$c_{44} \frac{\partial^2 w}{\partial x^2} + c_{44} \frac{\partial^2 w}{\partial y^2} + c_{33} \frac{\partial^2 w}{\partial z^2} + (c_{13} + c_{44}) \frac{\partial^2 u}{\partial x \partial z} + (c_{13} + c_{44}) \frac{\partial^2 v}{\partial y \partial z} = \tag{5}$$

(b) Initially Stressed Elastic Medium

In the absence of external forces, the equilibrium equations in the Cartesian coordinate system (x,y,z) for the unbounded medium with normal initial stress $S_{11} = -P$ along the horizontal direction (Fig.2) are

$$\begin{aligned} \frac{\partial s_{11}}{\partial x} + \frac{\partial s_{12}}{\partial y} + \frac{\partial s_{13}}{\partial z} - P \frac{\partial w_x}{\partial y} + P \frac{\partial w_y}{\partial z} &= 0, \\ \frac{\partial s_{21}}{\partial x} + \frac{\partial s_{22}}{\partial y} + \frac{\partial s_{23}}{\partial z} - P \frac{\partial w_x}{\partial x} &= 0 \end{aligned}$$

$$\frac{\partial s_{31}}{\partial x} + \frac{\partial s_{32}}{\partial y} + \frac{\partial s_{33}}{\partial z} + P \frac{\partial w_y}{\partial x} = 0 \tag{6}$$

where s_{ij} ($i, j = 1,2,3$) are the incremental stress components and w_x, w_y, w_z are the rational components given by

$$\begin{aligned} w_x &= \frac{1}{2} \left(\frac{\partial w}{\partial y} - \frac{\partial v}{\partial z} \right), \\ w_y &= \frac{1}{2} \left(\frac{\partial u}{\partial z} - \frac{\partial w}{\partial x} \right), \\ w_z &= \frac{1}{2} \left(\frac{\partial v}{\partial x} - \frac{\partial u}{\partial y} \right) \end{aligned} \tag{7}$$

where u, v and w are the displacement components.

The stress- strain relations for an initially stressed orthotropic elastic medium, with co-ordinate planes as planes of elastic symmetry, are

$$\begin{aligned} S_{11} &= B_{11}e_{11} + B_{12}(e_{22}+e_{13}), \\ S_{22} &= (B_{12}-P)e_{11} + B_{22}e_{22} + B_{23}e_{33}, \\ S_{33} &= (B_{12}-P)e_{11} + B_{23}e_{22} + B_{22}e_{33}, \\ S_{23} &= 2Q_1 e_{23} \\ S_{31} &= 2Q_2 e_{31} \\ S_{12} &= 2Q_3 e_{12} \end{aligned}$$

(8)

The incremental strain components e_{ij} ($i, j = 1,2,3$) are related with the displacement components (u,v,w) through the relations (4).

For the plane-strain deformation, parallel to xy -plane, in which the displacement components are independent of z and are of the type

$$u = u(x,y), \quad v = v(x,y), \quad w = 0.$$

The non-zero stresses for plane strain problem are

$$\begin{aligned} S_{11} &= B_{11}e_{11} + B_{12}e_{22} \\ S_{22} &= (B_{11} - P)e_{11} + B_{22}e_{22} \\ S_{12} &= 2Q_3e_{12} \end{aligned} \tag{8a}$$

where B_{ij} ($i,j=1,2,3$) and Q_3 are the incremental elastic coefficients and shear modulus, respectively. These incremental elastic coefficients are related to Lamé's coefficients of the isotropic unstressed state. For this case these are

$$\begin{aligned} B_{11} &= [\lambda + 2\mu(1 + \zeta)], & B_{12} &= (\lambda + 2\mu\zeta), \\ B_{21} &= \lambda, & B_{22} &= (\lambda + 2\mu), & Q_3 &= \mu \end{aligned} \tag{8b}$$

where $\zeta = \frac{P}{2\mu}$ is the initial stress parameter.

The equilibrium equations in terms of displacement components are

$$\begin{aligned} B_{11} \frac{\partial^2 u}{\partial x^2} + \left[Q_3 + \frac{P}{2} \right] \frac{\partial^2 u}{\partial y^2} + \left[Q_2 + \frac{P}{2} \right] \frac{\partial^2 u}{\partial z^2} + [B_{12} + Q_3 - \frac{P}{2}] \frac{\partial^2 u}{\partial x \partial y} + [B_{12} + Q_2 - \frac{P}{2}] \frac{\partial^2 u}{\partial x \partial z} = 0, \\ [B_{12} + Q_3 - \frac{P}{2}] \frac{\partial^2 u}{\partial x \partial y} + \left[Q_3 - \frac{P}{2} \right] \frac{\partial^2 v}{\partial x^2} + B_{22} \frac{\partial^2 v}{\partial y^2} + Q_1 \frac{\partial^2 v}{\partial z^2} + [B_{23} + Q_1] \frac{\partial^2 v}{\partial y \partial z} = 0, \\ [B_{12} + Q_2 - \frac{P}{2}] \frac{\partial^2 u}{\partial x \partial z} + [B_{23} + Q_1] \frac{\partial^2 v}{\partial y \partial z} + \left[Q_2 - \frac{P}{2} \right] \frac{\partial^2 w}{\partial x^2} + Q_1 \frac{\partial^2 w}{\partial y^2} + B_{22} \frac{\partial^2 w}{\partial z^2} = 0 \end{aligned} \tag{9}$$

III. Rectangular Irregularity

The origin of Cartesian coordinates is situated at $x=0$. When the rectangular irregularity is present in the elastic medium length $2a$ and depth h , the equation of irregularity at $x=\epsilon f(y)$ is

$$\epsilon f(y) = \begin{cases} h & \text{for } |y| \leq a \\ 0 & \text{for } |y| > a \end{cases}$$

and ϵ is a small parameter $\epsilon = \frac{h}{2a} \ll 1$

IV. Initially Unstressed Transversely isotropic Elastic Half-Space

(a) Normal Line-Load in An Irregular Transversely isotropic Elastic Half-Space

For the irregular-medium consisting of region $x < \epsilon f(y)$ (Medium I) and regions $x > \epsilon f(y)$ (Medium II) of identical elastic properties,

the displacement and stress components due to normal line load R_1 per unit length acting on z -axis, Madan [6] are,

Medium I

$$\begin{aligned} u^I(x,y) &= \frac{1}{2\pi} \int_{-\infty}^{\infty} i \left[[B|k| + C(x|k| - M_1^I)] e^{|k|x} \right] e^{-iky} dk \\ v^I(x,y) &= \frac{1}{2\pi} \int_{-\infty}^{\infty} k \left\{ B + C \left(x - \frac{1}{|k|} \right) \right\} e^{|k|x} \right] e^{-iky} dk \\ \sigma_{11}^I(x,y) &= \frac{1}{2\pi} \int_{-\infty}^{\infty} i \left\{ [Bk^2(C_{11} - C_{12}) + C(xk^2(C_{11} - C_{12}) + |k|(C_{12} - M_2^I C_{11}^I))] e^{|k|x} e^{-iky} \right\} dk \\ \sigma_{12}^I(x,y) &= \frac{1}{2\pi} \left(\frac{C_{11} - C_{12}}{2} \right) \int_{-\infty}^{\infty} k \left\{ [2B|k| + C(2|k|x - M_1^I)] e^{|k|x} \right\} e^{-iky} dk \end{aligned} \tag{11}$$

Medium II

$$u^H(x,y) = \frac{1}{2\pi} i \int_{-\infty}^{\infty} [-D|k| + G(x|k| - M_1^H)] e^{-|k|x} e^{-iky} dk$$

$$v^H(x,y) = \frac{1}{2\pi} k \int_{-\infty}^{\infty} \left[D + Gk \left(x + \frac{1}{|k|} \right) \right] e^{-|k|x} e^{-iky} dk \tag{13}$$

$$\sigma_{11}^H(x,y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} i \left[Dk^2 (C_{11} - C_{12}) + G(xk^2(C_{11} - C_{12}) - |k|(C_{12} - M_1^H C_{11})) \right] e^{-|k|x} e^{-iky} dk \tag{b}$$

$$\sigma_{12}^H(x,y) = \frac{1}{2\pi} \left(\frac{C_{11} - C_{12}}{2} \right) \int_{-\infty}^{\infty} k [-2D|k| + G(2|k|x - M_1^H)] e^{-|k|x} e^{-iky} dk \tag{14}$$

Boundary Conditions

For a normal line-load R_1 per unit length is acting vertically downwards on the interface irregularity $x = h$ along z -axis (Fig.1) Then the boundary conditions at $x=h$ are

$$u^I(h,y) = u^{II}(h,y),$$

$$v^I(h,y) = v^{II}(h,y),$$

$$\sigma_{12}^I(x=h,y) = \sigma_{12}^{II}(x=h,y)$$

$$\sigma_{11}^{II}(x=h,y) - \sigma_{11}^I(x=h,y) = -R_1 \delta(y)$$

Where $h = \epsilon f(y)$ and $\delta(y)$ is the Dirac - Delta satisfying the following properties:

$$\int_{-\infty}^{\infty} \delta(y) dy = 1, \quad \delta(y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} e^{-i\eta y} d\eta = 1$$

The closed form expressions for displacement at any point of an irregular transversely isotropic elastic half space due to normal line force acting at $(h,0)$ Madan [6] are

$$u^n(x,y) = \frac{R_0}{2\pi} \left\{ \left((k_2 M_1^I + k_3 M_1^H - 2k_1) \log(x^2 + y^2) - (k_2 + k_3) \frac{2x^2}{x^2 + y^2} \right) - \gamma \left(\frac{(\delta_1 - \delta_3)}{2} + (\delta_4 M_1^H - M_1^I) \right) \frac{2x^2}{x^2 + y^2} + (\delta_2 + \delta_4) \frac{x^2(x^2 - y^2)}{(x^2 + y^2)^2} \right\} \tag{15}$$

$$v^n(x,y) = -\frac{R_0}{2\pi} \left\{ 2(k_2 - 2k_1 + k_3) \tan^{-1} \frac{x}{y} + (k_2 + k_3) \frac{2xy}{x^2 + y^2} \right\} + \gamma \left\{ \left(\frac{\delta_1 + \delta_3}{2} (\delta_4 - \delta_2) \right) \frac{2xy}{x^2 + y^2} + (\delta_4 - \delta_2) \frac{4x^3 y}{(x^2 + y^2)^2} \right\}$$

(b) Tangential Line-Load in An Irregular Transversely isotropic Elastic Half-Space

Boundary Conditions

For a tangential line-load R_2 per unit length is acting in the positive y -direction on the interface irregularity $x = h$ along z -axis . Then the boundary conditions at $x=h$ are

$$u^I(h,y) = u^{II}(h,y),$$

$$v^I(h,y) = v^{II}(h,y)$$

$$\sigma_{11}^I(x=h,y) = \sigma_{11}^{II}(x=h,y)$$

$$\sigma_{12}^I(x=h,y) - \sigma_{12}^{II}(x=h,y) = -R_2 \delta(y) \tag{16}$$

where $h = \epsilon f(y)$ and $\delta(y)$ is the Dirac - Delta satisfying the following properties:

$$\int_{-\infty}^{\infty} \delta(y) dy = 1, \quad \delta(y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} e^{-i\eta y} d\eta = 1$$

The closed form expressions for displacement at any point of an irregular transversely isotropic elastic half space due to normal line force acting at $(h,0)$ Madan [6] are

$$u^t(x,y) = -\frac{R_2}{2\pi} \left\{ \left((k_1 + k_2 - k_3 M_1^I + k_4 M_1^H) \frac{2x}{x^2 + y^2} - ((k_3 + k_4) \frac{2x(x^2 - y^2)}{(x^2 + y^2)^2}) \right) - \gamma \left(\frac{2\delta_1(x^2 - y^2)}{x(x^2 + y^2)^2} + \frac{4x\delta_3(x^2 - 3y^2)}{(x^2 + y^2)^2} + \frac{2M_1^I \delta_3(x^2 - y^2)}{x(x^2 + y^2)^2} \right) \right\} \tag{17}$$

$$v^t(x,y) = -\frac{R_2}{2\pi} \left[\left((k_1+k_2-k_3+k_4) \frac{2x}{x^2+y^2} \right) + y \left(\left(\delta_1+\delta_2+\delta_3-\delta_4 - \frac{3x^2-y^2}{x^2+y^2} \right) + (k_4-k_3) \frac{4x^3y}{(x^2+y^2)^2} \right) \right]$$

(c) Inclined Line load

For inclined line load R_0 , per unit length, $R_1=R_0\cos \beta$, $R_2 = R_0 \sin \beta$

The displacement subjected to inclined line load can be obtained by superposition of the normal and tangential cases.

$$U^{inc}(x,y) = u^n(x,y) + u^t(x,y)$$

$$V^{inc}(x,y) = v^n(x,y) + v^t(x,y) \tag{18}$$

where $u^n(x,y)$, $v^n(x,y)$ & $u^t(x,y)$, $v^t(x,y)$ are the deformation due to a normal line load R_1 and a tangential line load R_2 respectively.

(d) Particular Case

By using equation (2), the corresponding results in isotropic elastic medium can be obtained from equation (11)-(15) & (17)-(18) as a particular case.

V. Initially Stressed Elastic Medium

Normal line-load

For infinite medium as consisting of region $x < h$ (Medium II) and region $x > h$ (Medium I), $h = \epsilon f(y)$.

The displacement and stress components for the medium I are Selim [16]

$$u^I(x,y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \left(-C_3 P_1^I e^{-\xi_1^I |\eta| x} - C_4 P_2^I e^{-\xi_2^I |\eta| x} \right) e^{-i\eta y} d\eta$$

$$v^I(x,y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \left(C_3 e^{-\xi_1^I |\eta| x} + C_4 e^{-\xi_2^I |\eta| x} \right) e^{-i\eta y} d\eta$$

$$S_{11}^I = \frac{1}{2\pi} \int_{-\infty}^{\infty} \left(H_1^I C_3 e^{-\xi_1^I |\eta| x} + H_2^I C_4 e^{-\xi_2^I |\eta| x} \right) e^{-i\eta y} d\eta \tag{19}$$

$$S_{12}^I = \frac{1}{2\pi} \int_{-\infty}^{\infty} Q_3^I \left(-H_1^I C_3 e^{-\xi_1^I |\eta| x} - H_2^I C_4 e^{-\xi_2^I |\eta| x} \right) e^{-i\eta y} d\eta \tag{19}$$

and for the medium II are

$$u^{II}(x,y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \left(C_1 P_1^{II} e^{\xi_1^{II} |\eta| x} + C_2 P_2^{II} e^{\xi_2^{II} |\eta| x} \right) e^{-i\eta y} d\eta$$

$$v^{II}(x,y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} \left(C_1 e^{\xi_1^{II} |\eta| x} + C_2 e^{\xi_2^{II} |\eta| x} \right) e^{-i\eta y} d\eta$$

$$S_{11}^{II} = \frac{1}{2\pi} \int_{-\infty}^{\infty} \left(H_1^{II} C_1 e^{\xi_1^{II} |\eta| x} + H_2^{II} C_2 e^{\xi_2^{II} |\eta| x} \right) e^{-i\eta y} d\eta$$

$$S_{12}^{II} = \frac{1}{2\pi} \int_{-\infty}^{\infty} Q_3^{II} \left(-H_1^{II} C_1 e^{\xi_1^{II} |\eta| x} + H_2^{II} C_2 e^{\xi_2^{II} |\eta| x} \right) e^{-i\eta y} d\eta \tag{20}$$

where

$$(\xi_{K}^L)^2 = \frac{B_0^L \pm \sqrt{(B_0^L)^2 - 4C_0^L}}{2}, B_0^L = \frac{(B_{11}^L)^2 + N_2^L N_3^L - (N_2^L)^2}{B_{11}^L N_3^L}, C_0^L = \frac{N_2^L}{N_3^L}$$

$$N_1^L = \mu^L(1+\zeta), N_2^L = [\lambda^L + \mu^L(1+\zeta)], N_3^L = \mu^L(1-\zeta), B_{11}^L = [\lambda^L + 2\mu^L(1+\zeta)]$$

$$B_{22}^L = [\lambda^L + 2\mu^L], Q_3^L = \mu^L, B_{12}^L = [\lambda^L + 2\mu^L \zeta], P_k^L = i \frac{\xi_k^L N_2^L}{(\xi_k^L)^2 B_{11}^L - N_1^L}$$

$$H_k^L = B_{11}^L P_k^L \xi_k^L |\eta| - i B_{12}^L \eta, H_k^L L = \xi_k^L |\eta| - i P_k^L \eta, L = I, II \text{ and } K = 1, 2. \tag{21}$$

Boundary conditions

When a normal line-load F_0 per unit length, is acting vertically downwards on the interface irregularity $x = h$ along z -axis (Fig.2). Then the boundary conditions at $x = h$ are

$$u^I(h,y) = u^{II}(h,y),$$

$$v^I(h,y) = v^{II}(h,y)$$

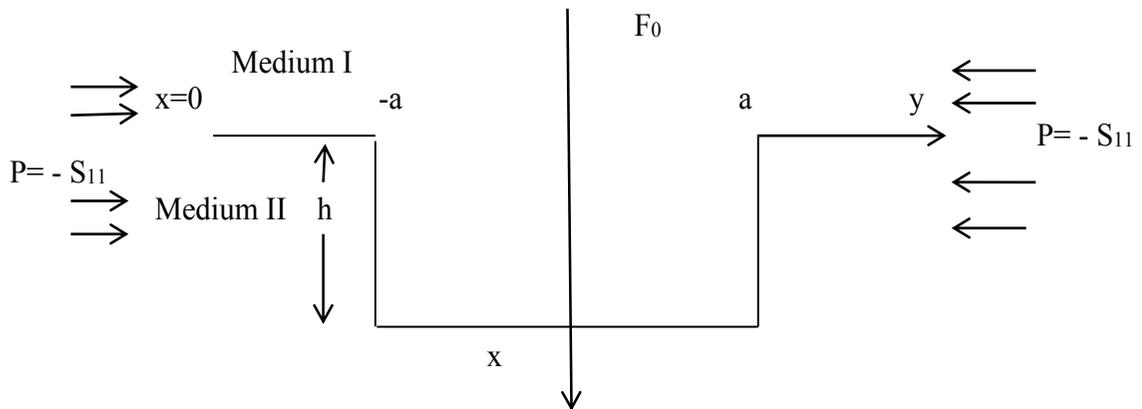
$$S_{12}^I(x=h,y) = S_{12}^{II}(x=h,y)$$

$$S_{11}^{II}(x=h,y) - S_{11}^I(x=h,y) = -F_0 \delta(y) \tag{22}$$

where $h = \epsilon f(y)$ and $\delta(y)$ is the Dirac-delta satisfying the following properties.

$$\int_{-\infty}^{\infty} \delta(y) dy = 1, \quad \delta(y) = \frac{1}{2\pi} \int_{-\infty}^{\infty} e^{-i\eta y} d\eta = 1$$

The expressions for displacement and stress at any point of an initially stresses, irregular, isotropic elastic half-space due to normal line force acting at $(h,0)$ are



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